MICROSTRUCTURAL CHARACTERISTICS OF BIOSOURCED CONCRETES REINFORCED WITH SUGAR CANE BAGASSE FIBERS: CASE OF ETTRINGITE FORMATION AND C-S-H PROPAGATION

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Abstract: In this work, we studied ettringite formation, C-S-H propagation, and the thermomechange properties of sugarcane bagasse (SBC) fiber-reinforced concrete with a maximum dosage of 0.17% by weight. Scanning electron microscopy (SEM), thermogravimetric analysis (TGA), and infrared (IR) analyses were performed. TGA results showed bio-based concrete resistant to temperatures exceeding 500°C. TGA results revealed thermal stability of bio-based concrete. SEM analysis showed ettringite and C-S-H propagation without altering mechanical properties. These results are consistent with those obtained by other authors.

11 Keywords: Concrete, bagasse fiber, thermogravimetric analysis, ettringite, C-S-H, SEM.

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1.Introduction

Replacing traditional building materials with bio-based alternatives with low thermal conductivity (or high thermal resistance) has become an attractive solution for improving the thermal efficiency and reducing the carbon footprint of buildings [12]. Bio-based materials possess hygroscopic properties that make them effect to as thermal insulators and contribute to a healthier and more comfortable living environment. It is in this sense that the development of bio-based concrete occupies an important place [1][3]. Plant-based products offe 2.n ecological and renewable option due to their sustainability criteria, such as local availability and low processing costs. The thermal resistance of conventional mortars was improved by 16% and 36% with the addition of 10% and 20% cork [4]. When concrete is subjected to an increase in temperal by a various physical and chemical transformations occur [5]. Sugarcane bagasse, incorporated into the concrete mat 3, reacts with the free CH present in the concrete to form the C-S-H network, strengthening the strength of the concrete [6]. Thermogravimetric analysis (TGA) is a widely used technique feather study of materials. It allows the determination of different hydrates and carbonates. TGA allows understanding material-specific reactions such as dihydroxylation, decarbonization, decomposition or changes in the crystalline structure [7]. Our work aims to study the microstructural characteristics of bio-sourced concretes from sugarcane bagasse fibers using TGA, SEM and IR on all concretes.

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3. Corracterization Methods 3.1. Microstructural Observations by Scanning Electron Microscopy (SEM)

Microstructure was observed by scanning electron microscopy (SEM) on fresh concret 26 agments from cylindrical samples after 28 days of storage. The instrument used was a Hitachi TM3030Plus with a beam accelerating voltage of 20 keV and a probe current of 150 to 250 pA. Before being subjected to the SEM, the samples were metallized with gold after drying at 40°C. The microstructures observed by SEM of the biofiber-reinforced concrete (SBC-15), polypropylene concrete (PC), and refe 23 e concrete (OC) are presented in Figures 1, 2, 3, and 5 below.
3.2. Ther mogravimetric Analysis (TGA)

The analysis was performed on powdered concrete fragments obtained by grinding to a thickness of less than 315 mm, from

We used the materials and methods described in our previous article [8].

cylindric 13 amples after 28 days of storage. The inst 10 ent used was the NETZSCH Tarsus TG209F3 model. Measurem began at an initial temperature of 30°C and ended at 600°C, with a heating rate of 10°K/min. The NETZSCH pellets (25 μL) used are made of aluminum and can withstand a maximum temperature of 600°C. 3.3. Infrared Spectrometry (IR 25

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This is an analytical technique used to determine the chemical composition and structure of materials. The analysis was performed on fresh concrete fragments from cylindrical samples after 28 days of storage. The instrument used was the Spectrum Two FT-IR Spectrometer. Analyses were performed on residues from the ATG.

50 51 52 4. Results and Discussion 53

4.1. Microstructural Observations by Scanning Electron Microscopy (SEM)

54 4.1.1. Ordinary Concrete (OC)

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Figure 1 shows the microstructures of the OC samples. Concrete hydrates such as ettringite needles and hydrated calcium silicate (C-S-H) can be observed.

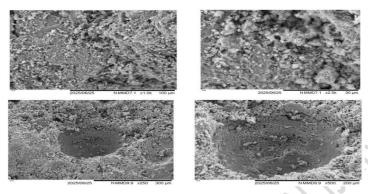


Figure 1: SEM of OC concrete

Secondary ettrigate formation is closely related to the transport of pore fluids within the concrete mass. The deterioration of a large number of concrete structures in contact with sulfate-free environments and related to secondary ettringite formation is currently one of the major research problems, very controversial for researchers sustainable materials [9]. Research on this topic suggests that the explanation for this phenomenon lies in the rapid absorption of sulfates by calcium silicate hydrate (C-S-H), thus preventing the usual formation of ettringite at an early stage. Later, the release of these sulfates into the pore fluid of the concrete causes the crystallization of ettringite. Expansion phenomena, existing cracks, and diffusing processes within the material play a key role throughout this process [10]. Sulfate can be combined by the C-S-H gel at an early stage of hydration and released at a later stage. The sulfate required for secondary ettringite formation comes from hydrated Portland cement. Concrete deterioration due to delayed ettringite formation can be attributed to an increase in temperature during concrete curing [10].

4.1.2. Sugarcane bagasse fiber concrete (SBC-0.15) not subjected to thermal stress

74 Figure 2 below shows the SEM of bagasse fiber concrete made with SBC-0.15.

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77 SEM images of

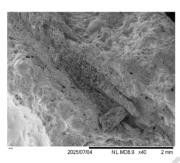
Figure 2: SEM of SBC-0.15

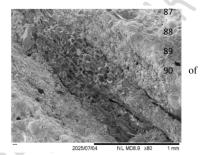
77 SBC-0.15 confirm the

porous structure and water absorption capacities of the fiber. This allows good adhesion with the concrete matrix. Ettringite and hydrates appear less compared to polypropylene fiber concrete. This justifies good porosity of the fiber. Bagasse fiber does not trap air as is the case with polypropylene fiber which has better porosity. Many cavities can be observed, probably due to the evaporation of water absorbed by the fibers during the pre-soaking phase or related to air trapped during mixing [11][12].

4.1.3. Sugarcane bagasse fiber concrete (SBC-0.15) subjected to thermal stress

Figure 3 below shows the SEM of SBC-0.15 subjected to thermal stress





1000°C in a CERADE C86 –160 C2 electric

Figure 3: SEM of SBC-0.15 subjected to thermal stress

kiln. A heating phase at a speed of 1°C/min up to

the predefined temperature T°C. The kiln can reach a temperature of 1300°C, the temperature

is predefined in advance during a technical adjustment of the device. A fan associated with

the heating allows the temperature to be regulated and homogenized by the circulation of air

between the heating resistors. Figure 4 below shows the electric furnace containing the

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Figure 4: Electric furnace containing the concrete

The bagasse fiber was able to resist in this aggressive environment [13]. We can observe the porous network of the bagasse fiber and an expansion of the concrete matrix due to thermal stress. In Figure 4, the microcracks can be explained by the expansion of the sugarcane bagasse fiber during melting, which generates stresses and consequently the formation of 110 cracks. The presence of the fibers modifies the geometry and morphology of the network

Porous. This phenomenon is due to the loss of water initially contained in the C-S-H hydrates

112 [14][15].

4.1.4. Polypropylene (PC) fiber concrete

Figure 5 shows the SEM of concrete made with polypropylene fibers.





Figure 5 : SEM of PC at room temperature

Calcium-silica-hydrate (C-S-H) gels and ettringite generally appear in lighter shades. Hydrates and ettringites are found to cover all concrete fibers and layers, acting as a binder, which can lead to pore-blocking, reduced permeability, and reduced porosity. Polypropylene fibers can also be said to trap air in the mix, increasing void volume and not improving porosity. As a consequence, this can lead to a decrease in compressive strength [16] [17].

4.2. Concrete Thermal Stability Measurements: Thermogravimetric Analysis (TGA)

The analysis was conducted on powdered concrete fragments obtained by grinding to a thickness less than 315 mm, from cylindrical samples after 28 days of 30 orage. The device used is the NETZSCH Tarsus TG20000 model. Measurements begin at an initial temperature of 30°C and end at a temperature of 600°C, with a heating rate of 10°K/min. The NETZSCH pellets (25 µL) used are made of aluminum and can withstand a maximum temperature of 600°C.

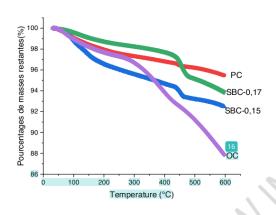


Figure 6: ATG curves of the different selected concretes

 Figure 6 shows typical mass loss curves during a temperature increase of 10°C/min. The first derivative signals (DTG) for the different concretes are given in Figures 7, 8 and 9

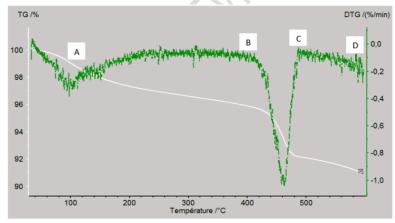


Figure 7: DTG curve of OC concrete. The recorded curve for the ATG of OC is superimposed for comparison purposes (in white).

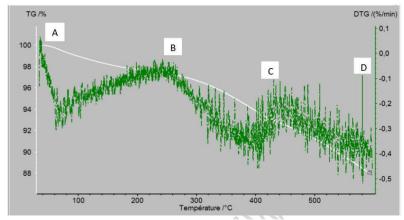


Figure 8: DTG curve of SBC-15 concrete. The recorded curve for the ATG of SBC-15 is superimposed for comparison (in white)

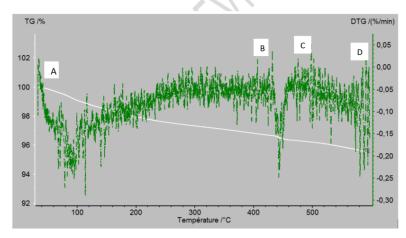


Figure 9: DTG curve of PC concrete. The recorded curve for the ATG of PC is superimposed for comparison purposes (in white)

Several degradation phases are observable on these curves, closely related to the composition of the concretes. Regarding ordinary concrete (OC), three distinct stages are observed, revealing the products generated by the hydration reactions between cements and water [18]. In Figure 7, the first sign of mass loss is perceptible between 35 and 135 °C, as indicated by

peak A on the DTG curve. This loss results from both the evaporation of capillary water and the elimination of water associated with ettringite hydrate. The mass loss observed between 400 and 500 °C can attributed to the release of water present in hydrated calcium silicates (C-S-H), and to the dehydroxylation of portlandite (Ca(OH2). Finally, the mass loss in the temperature range of 500 to 600 °C, i.e. between C and D, corresponds to the decarbonation of calcium carbonate; it is probably linked to poorly crystallized carbonate compounds [19].

151 Regarding the formulations of concretes containing sugarcane bagasse fibers, specific characteristics can be highlighted. In Figure 8, the TGA results indicate that the overall 152 reight loss observed between 400 and 500 °C is significantly higher than that of OC concrete 153 and that this difference increases as the fiber content increases. This can be attributed to the 154 155 evaporation of excess water used to pre-soak the fibers and preserve the workability of the 156 concretes, as well as to the degradation of bagasse fibers under the effect of the kiln heat. The degradation rends of bio-based concretes, as illustrated by the DTG curve of SBC-0.15 157 158 (Figure 8), are similar to those previously observed for OC concrete. This indicates the 159 presence of the same cement hydrates, such as ettringite, portlandite and calfum carbonate 160 hydrates. A notable difference is the slowing of the peak at points C and D, indicating a higher relative proportion of calcium carbonate. 161

This deduction can be attributed to the higher porosity, demonstrated by SEM analysis, which 162 163 makes these bio-based concretes more sensitive to carbonation. The amplitude of the DTG 164 peak at point A in bio-based concretes is significantly higher than that of ordinary OC 165 concrete. This can be explained due to the evaporation of residual water from sugarcane bagasse fibers. The amplitude of the DTG peak at points B and C related to portlandite is 166 broader, compared to that in Figure 6 of ordinary concrete. This is explained by the 167 incorporation of sugarcane bagasse fibers. This result suggests that the sugar molecules 168 169 extracted from sugarcane bages fibers had no negative impact on cement hydration. Alkali-170 pretreated sugarcane bagasse fibers contain low hemicellulose content compared to untreated 171

Regarding the ATG of PC concrete, the lack of porosity of the polypropylene fibers explains 172 why the PC curve has the best result out of all the curves in Figure 6. In Figure 9, we observe 173 174 a first weight loss at peak A, which is due to the evaporation of water from the capillary voids, which represent the space not occupied by the hydrated cement during the hydration 175 176 process. Between 100 and 200°C, the polypropylene fibers melt very quickly, causing 177 channeling, thus releasing heat pressure in the concrete. Observing Figure 9, between 200 and 500°C, that is, between point A and point C, the weight loss continues due to the loss of water 178 in the concrete, as well as the subsequent decomposition of the polypropylene fibers. Between 179 points B and C, the loss rate dropped sharply. This increase is due to the decomposition of 180 181 calcium hydroxide in the cement paste as well as the decomposition of the C-S-H phase of 182 ettringite, followed by the formation of beta-dicalcium silicate (b-C2S) [21].

4.3. Infrared Spectrometry (IR)

Figure 10 below shows the IR transmission spectra of the different concretes analyzed after

185 TGA.

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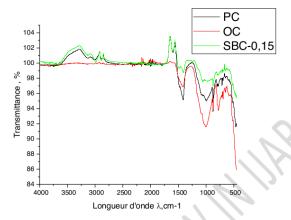


Figure 10: IR curves of different concretes

The results of SBC-0.18 bio-fibered concrete show different peaks between 2000 cm-1 and 1500 cm-1. This peak is associated with the O-H stretching vibration observed in cellulose, hemicelluloses and lignin which are the main sugar compounds [22]. The peaks in the SBC-0.15 curve are attributed to the asymmetric and symmetric C-H stretching vibrations of C12 in cellulose and hemicelluloses, respectively. Further, we can add that the two peaks are attributed to the O-H bending vibrations of water absorbed by cellulose and the C=O stretching of carboxylic acid and ester, respectively [23]. The IR spectroscopy results confirm the SEM images and TGA curves of 10-fibered concretes. They show that sugarcane bagasse fiber contains high concentrations of C, H, and O. These elements are combined into different functional groups characterized by different stretched wavenumbers, such as hydroxyl (O-H), hydrocarbon (C-H), and carbonyl (C-O) groups.

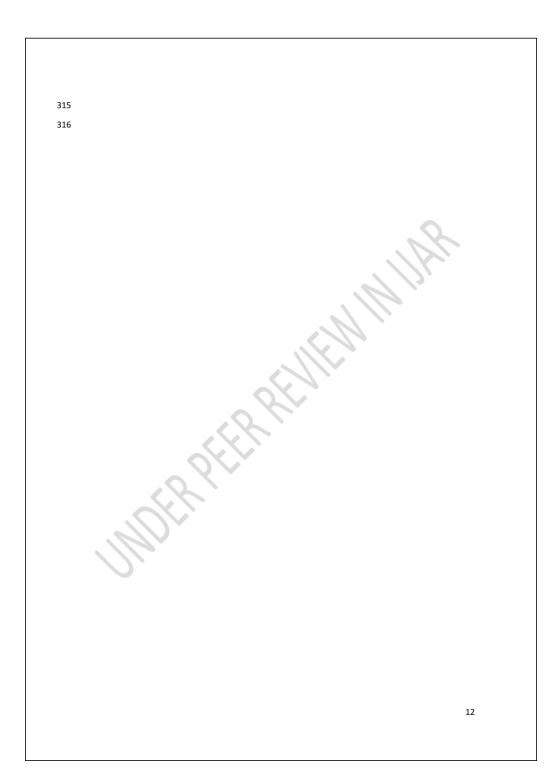
5. Conclusion

In this study, we performed a microstructural characterization of sugarcane bagasse fiber-reinforced concretes. We performed SEM, TGA, and IR on all concretes. The results of thermogravimetric analyses at 600°C of the bio-based bagasse fiber concretes are very similar to those of the polypropylene fiber concreted SEM observations of the bio-based concretes show the propagation of ettringite and C-S-H. This propagation does not affect the thermomechanical strength of the concretes. In all SEM observations, the bagasse fiber is integrated into the concrete matrix, which appears to provide a satisfactory environment for the formation of ettringite and the release of C-S-H. The porous nature of the sugarcane bagasse fiber prevents the propagation of air bubbles, which is not the case for the polypropylene fiber. This is not the case for polypropylene fiber. The IR results of bio-based concrete confirm the TGA findings. Additional experimental evidence is needed to study the propagation of ettringite and C-S-H. Sudden cooling of the concrete matrix can create

- 213 favorable conditions for matrix deterioration and ettringite formation. Sugarcane bagasse fiber
- 214 has very good incorporation qualities in concrete. It can therefore replace polypropylene fiber.
- 215 It withstands high temperatures without significantly altering its mechanical properties. This
- 216 study once again confirms its status as an ecological material for green, sustainable, and
- optimizable construction, as advocated by the United Nations.
- 218 Acknowledgments
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- 221 Conflict of Interest
- The authors declare no conflict of interest regarding the publication of this article.
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